

Traffic Signals Phasing and Timing

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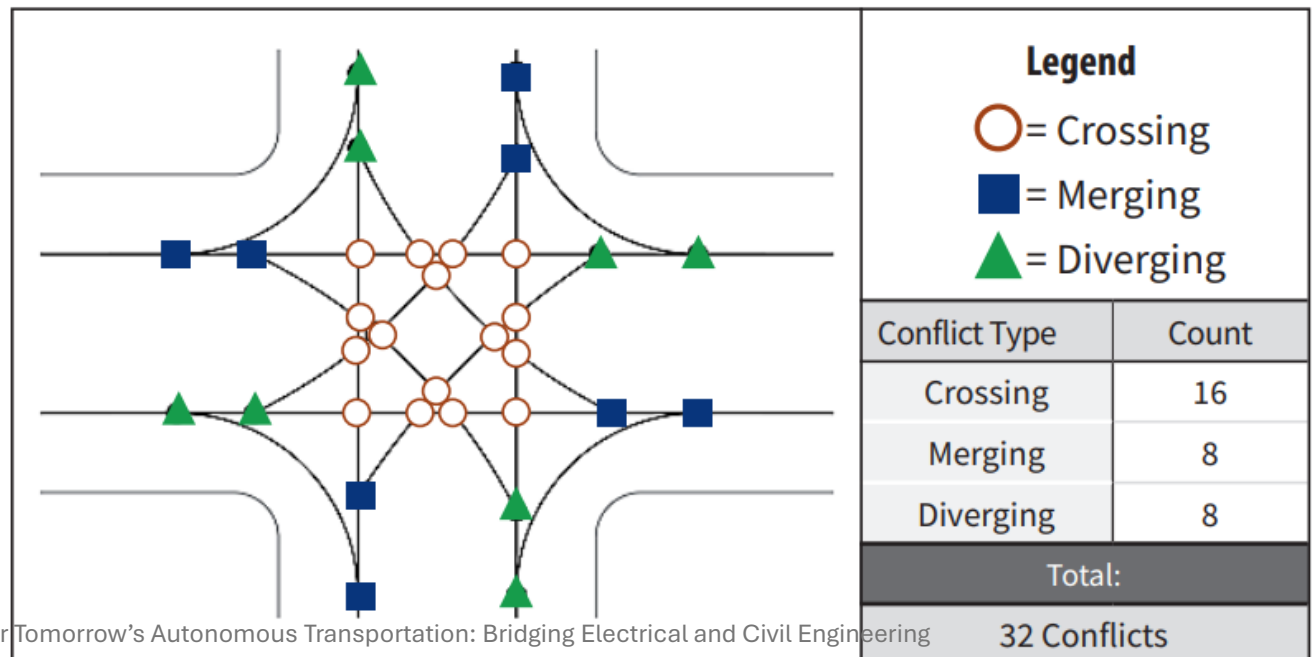
Learning Objectives

- 1 **Define and explain** key traffic-signal terminology, phasing concepts, and intersection control principles.
- 2 **Differentiate** between signal operation types and left-turn treatments.
- 3 **Interpret** typical signal-head configurations and their implications for safety and operations.
- 4 **Apply** formulas for yellow and all-red intervals to determine safe change and clearance times.
- 5 **Compute** critical flow ratios and **determine** appropriate cycle lengths.
- 6 **Develop** a basic signal timing plan including phase selection, cycle length, and green time allocation.

Introduction

- Intersections: The most complex location within transportation network.
 - 32 conflict points.

Conventional Intersection: Conflict Points

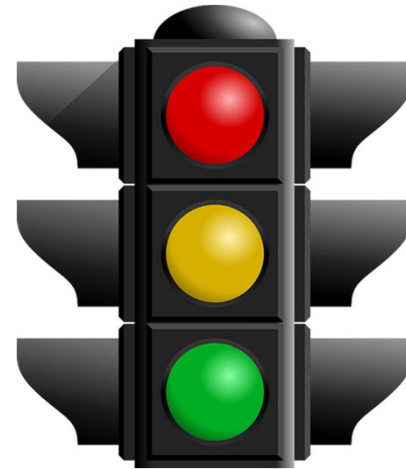


Introduction

- Three levels of intersection control:
 - Level I: Basic rules of the road.
 - Level II: YIELD or STOP signs
 - Level III: Traffic signals
- Which level to choose?
 - Driver's ability to avoid conflicts
 - See potential conflicts
 - Traffic volumes allow safe movement

Introduction

- Signalized Intersections:
 - Provide safe and efficient movement of veh/peds/bikes through the intersection.
 - Must be warranted.
- Important Concepts:
 - Time allocation
 - Phasing (left turns)

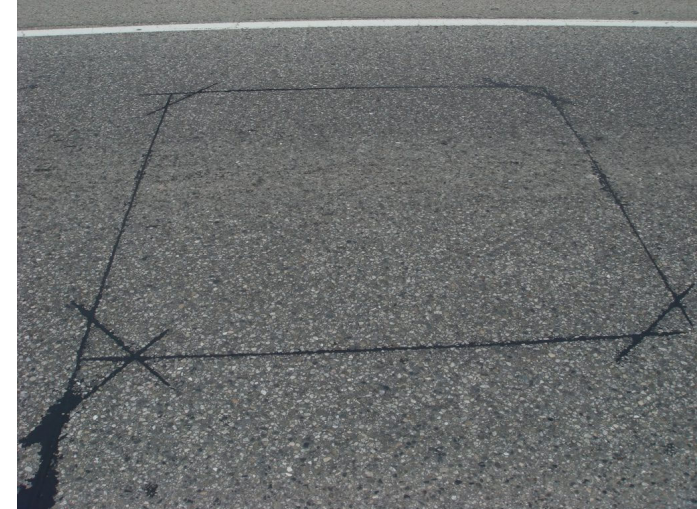


Traffic Signals: Terms

- **Cycle:** one complete rotation through all of the indications.
 - Cycle length: time to complete one full cycle (sec)
- **Interval:** period of time (sec) during which no signal indication changes.
 - Green (G_i): time (sec) when green is on
 - Change (Y_i): “yellow indication”
 - Clearance (AR_i): “all-red”
 - Red (R_i): time (sec) when red is on
- **Phase:** a set of intervals

Traffic Signals: Operations

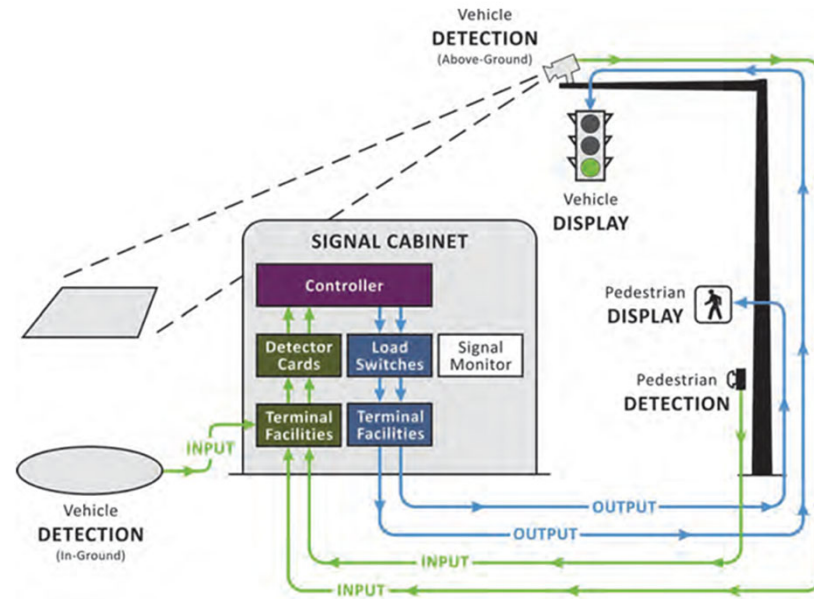
- **Pre-timed:**
 - Cycle, phase, intervals all predetermined.
 - Fixed-time of day, am/pm, off peak, weekend
- **Semi-actuated:**
 - Detectors only on minor approaches.
 - “Occupancy” sends a call for service.
- **Fully-actuated**
 - Detectors on all approaches
- **Demand Responsive**
 - Completely dynamic



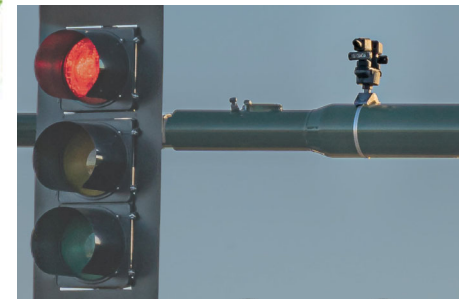
Traffic Signals: Equipment

- **Detection:**

- Inductive loop
- Cameras
- Radar
- Acoustic
- Pressure plates
- Push buttons



- Emergency Vehicle Preemption (opticom)

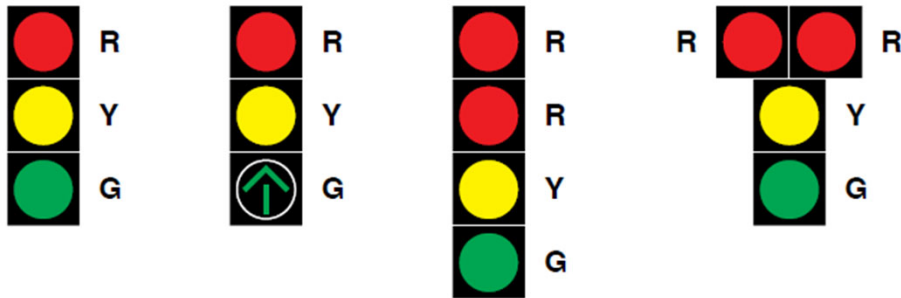


Traffic Signals: Left-Turn Treatments

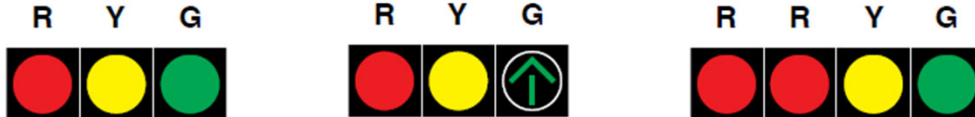
- **Permitted:**
 - Made across an opposing flow
- **Protected:**
 - Made without an opposing flow
- **Compound:**
 - Protected-Permitted

Traffic Signals: Configurations

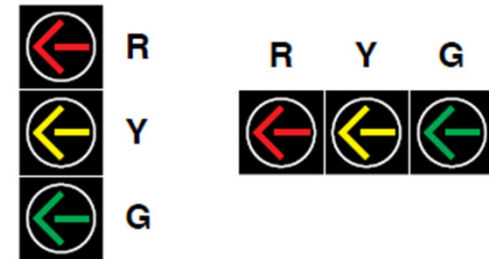
A – Vertical signal faces



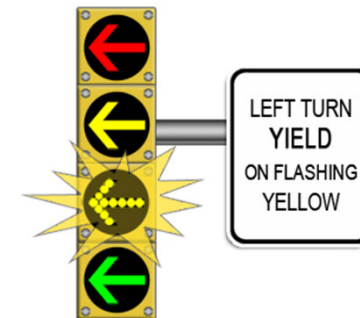
B – Horizontal signal faces



Permitted Left-Turns



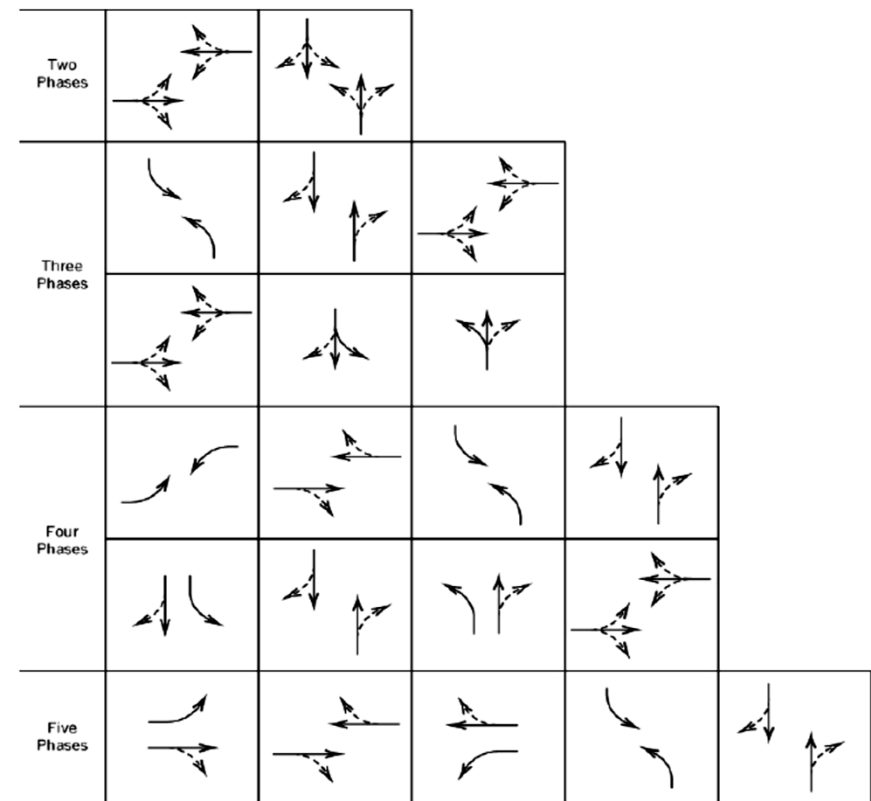
Protected Left-Turns



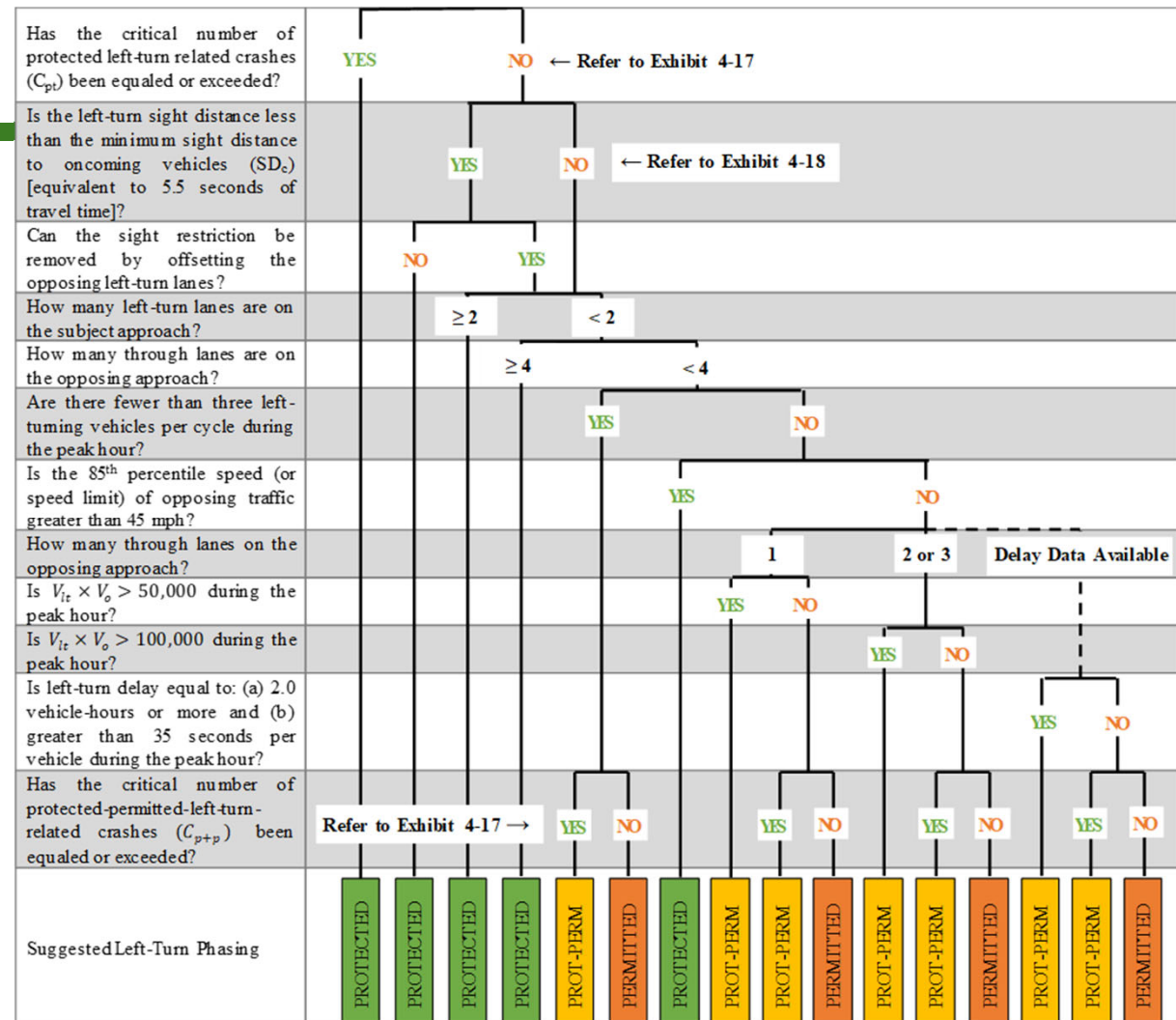
Protected/ Permitted Left-Turns

Select Signal Phasing

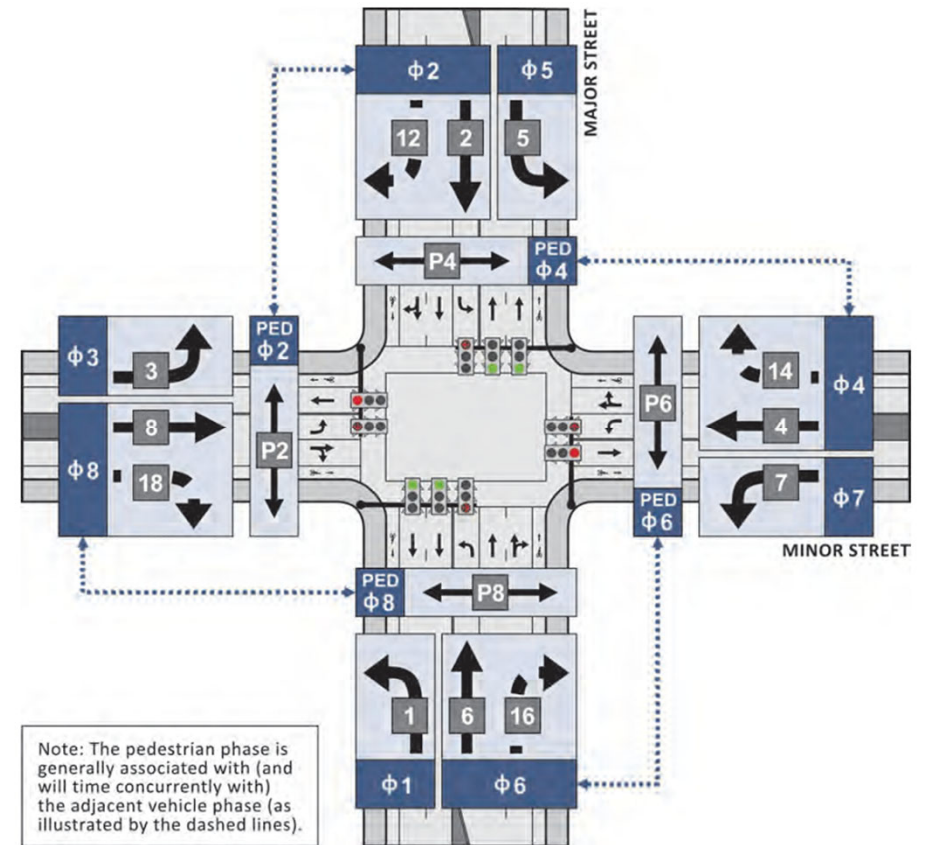
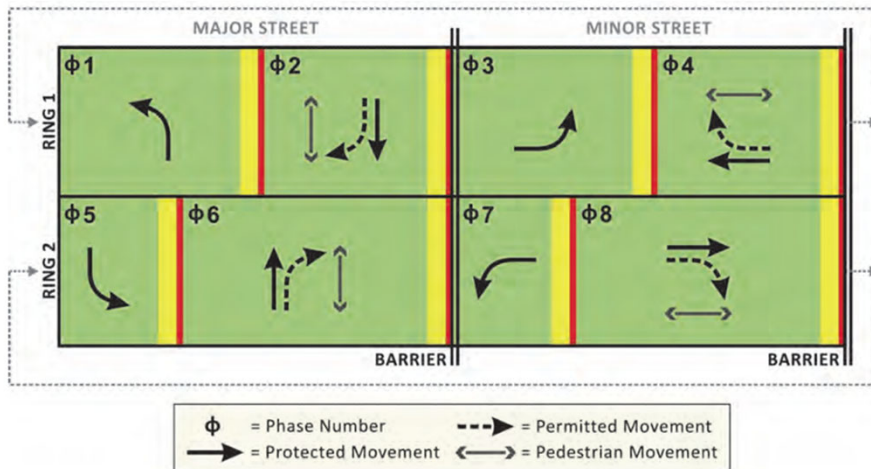
- Minimize number of phases to minimize lost times
 - 3 to 5 seconds total lost time per phase
- Determinant: **Left-Turns**



*(Adopted from “Signal Timing Manual”,
NCHRP, Second Edition, 2015)*



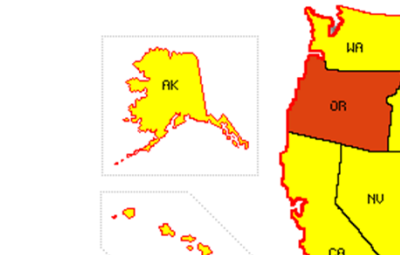
Numbering Movements and Phases

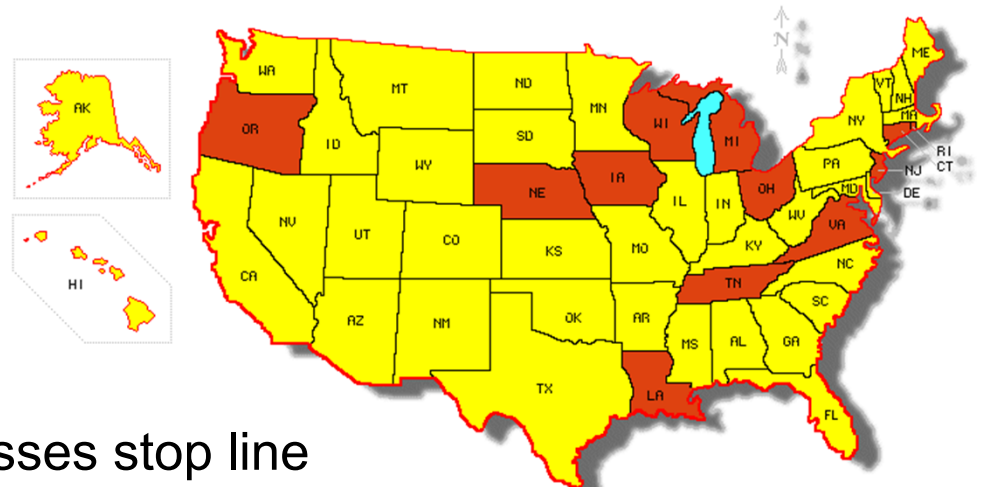
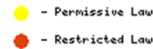


Signal Timing Plan

- Vehicular Signal Needs
 - Determine “Y+AR”
 - Determine V_c
 - Determine L_t
 - Determine C_{des}
 - Allocate effective green
- Pedestrian Signal Needs

Change Interval

- Indication: Circular Yellow (CY)
 - Purpose: To warn drivers that signal indication (right-of-way) is about to change from CG to CR.
 - Laws
 - Permissive
 - Can enter on CY if front axle crosses stop line
 - Restrictive
 - Don't enter on CY unless unsafe to stop
 - Can't be in intersection on CR
- 



Change Interval

- Duration of Yellow

- No national standard
- Adequate time for a driver to safely/conveniently stop [MUTCD: 3-6 sec]
- Rule of thumb: ~10% of speed limit
- ITE Recommendation:

$$Y = t_r + \frac{V}{2a + 2g_r G}$$

- t_r = driver perception/reaction time (~ 1.0 sec)
- V = vehicle speed (ft/s)
- a = vehicle deceleration (~ 10 ft/s²)
- g_r = acceleration due to gravity (32.2 ft/s²)
- G = grade (% divided by 100)

Clearance Interval (All-Red)

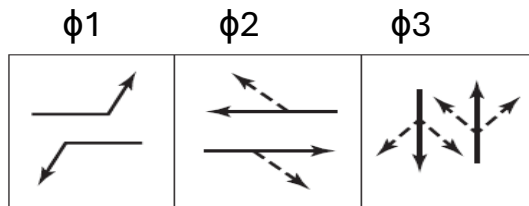
- If a vehicle enters at the end of the yellow, allow that vehicle to clear before conflict.
- Duration of AR
 - No national standard
 - Rule of thumb: ~ 1 sec
 - ITE Recommendation:

$$AR = \frac{w + l}{V}$$

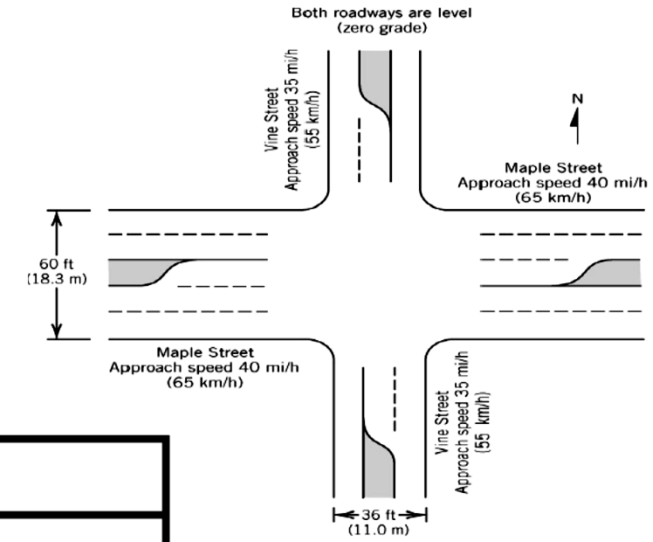
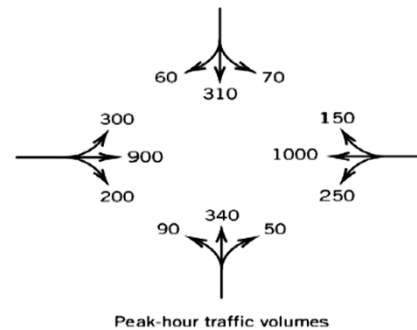
- w = width of street that vehicle must travel (ft)
- l = vehicle length (20 ft)

Determine Critical Flow Ratios

- Example



Phase Diagram



Phase	ϕ_1	ϕ_2	ϕ_3
Sat. Flows	EBL: 1750 vph WBL: 1750 vph	EBTR: 3400 vph WBTR: 3400 vph	SBTR: 1800 vph NBTR: 1800 vph SBL: 450 vph (permitted) NBL: 475 vph (permitted)
v/s	EBL: $300/1750 = 0.171$ WBL: $250/1750 = 0.143$	EBTR: $1100/3400 = 0.324$ WBTR: $1150/3400 = 0.338$	SBTR: $370/1800 = 0.206$ NBTR: $390/1800 = 0.217$ SBL: $70/450 = 0.156$ NBL: $90/475 = 0.189$

Calculate Cycle Length

- Minimum Cycle Length

$$C_{\min} = \frac{L \times X_c}{X_c - \sum_{i=1}^n \left(\frac{v}{s} \right)_{ci}}$$

- $C_{\min}$ = Minimum necessary cycle length (sec)
 L = Total lost time for cycle (sec)
 X_c = Critical v/c ratio (intersection utilization)
 $(v/s)_{ci}$ = Flow ratio for critical lane group i
 n = Number of critical lane groups

Calculate Cycle Length

- Optimum Cycle Length

$$C_{opt} = \frac{1.5L + 5.0}{1.0 - \sum_{i=1}^n \left(\frac{v}{s} \right)_{ci}} \quad (\text{Webster})$$

C_{opt} = Optimum cycle length to minimize delay (sec)

L = Total lost time for cycle (sec)

$(v/s)_{ci}$ = Flow ratio for critical lane group i

n = Number of critical lane groups

Allocate Green Time

- How much green time should be allocated to each phase
- Multiple strategies.
 - One of the most popular and simplest is to distribute the green time so that v/c ratios are equalized for the critical lane groups.

$$g_i = \left(\frac{v}{s}\right)_{ci} \left(\frac{C}{X_i}\right)$$

g_i = Effective green time for phase i (sec)

C = Cycle length (sec)

X_i = v/c ratio for critical lane group i

$(v/s)_{ci}$ = Flow ratio for critical lane group i

Check Pedestrian Crossing

- The minimum pedestrian green time is given by:

$$G_{ped} = 3.2 + \frac{L}{S_{ped}} + (0.27 N_{ped}) \quad \text{for } W_E \leq 10 \text{ ft}$$

$$G_{ped} = 3.2 + \frac{L}{S_{ped}} + \left(2.7 \frac{N_{ped}}{W_E} \right) \quad \text{for } W_E > 10 \text{ ft}$$

where

- G_{ped} = pedestrian green time
- W_E = effective crosswalk width
- L = crosswalk length (ft)
- S_{ped} = pedestrian walking speed (3.5 – 4.0 ft/s)
- N_{ped} = number of pedestrians

For each crosswalk confirm: $G_{ped} < G$. (If $G_{ped} > G$, then set $G = G_{ped}$)

References

- Mannering, F., and S. Washburn (2020). Principles of Highway Engineering and Traffic Analysis, Seventh Edition, John Wiley & Sons
- Roess, R.P., Prassas, E.S., and McShane, W.R. (2019) Traffic Engineering, 5th Edition.
- Traffic Signal Timing Manual, 2nd Edition, National Cooperative Highway Research Program Report 812, Washington, D.C., 2015
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